

**DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA**

INTERDEPARTMENT CORRESPONDENCE

FILE: BHNLB-9073-00(016) & BRNLB-9073-00(018) **OFFICE:** Engineering Services
 Fulton County
 P. I. Nos.: 752086 & 752560
 CS 999/Spring Street Viaduct Replacement over **DATE:** March 17, 2009
 CSX Railroad and Southern Railroad

FROM: Ronald E. Wishon, Acting Project Review Engineer *REW*

TO: James B. Buchan, P.E., State Urban Design Engineer
 Attention: Nicoe Alexander

SUBJECT: IMPLEMENTATION OF VALUE ENGINEERING STUDY ALTERNATIVES

Recommendations for implementation of Value Engineering Study Alternatives are indicated in the table below. Incorporate alternatives recommended for implementation to the extent reasonable in the design of the project.

ALT No.	Description	Savings PW & LCC	Implement	Comments
Structural Steel (A)				
A-2	Use a single concrete box beam span for the CSX span 15 - 17.	Proposed= \$445,000 Actual= (-\$871,000)	No	Would do away with access to buildings and introduce additional ROW impacts. Would add cost to the project.
Superstructure Concrete AA (B)				
B-1	Use three 11-foot lanes with curb and gutter on both sides for Spring Street.	Proposed= \$832,000 Actual= \$40,000	No	Would not match existing bridge on either end.
B-2	Use one 14-foot sidewalk on the West side of Spring Street north of the section built in 1995, in lieu of two 10-foot sidewalks.	Proposed= \$1,296,000 Actual= \$83,000	No	Current need for sidewalk on both sides. City of Atlanta requiring two 10-foot sidewalks.

Superstructure Concrete AA (B) Continued				
B-5	Use 6-inch thick sidewalks on the bridge in lieu of the proposed 7-inch sidewalk recommended to match the existing sidewalk constructed in 1995.	\$88,700	No	Will cause cross slope to increase greater than 2% which would not meet ADA requirements. Would require profile redesign and would cause impacts to businesses front doors. Matches existing 7-inch sidewalks.
B-6	Use Class B concrete for sidewalks on the bridge.	\$426,500	No	Class B concrete is not as durable as Class AA. Cost analysis did not provide for rebar in the Class B concrete, therefore cost savings would be diminished. See attached memo dated 3-6-09.
Drilled Caissons (C)				
C-1	Use more, smaller diameter drilled caissons in lieu of larger diameter caissons.	Design Suggestion	No	Would add cost to the project.
Other (F)				
F-1	Rather than total reconstruction and improvements on Madison Street, provide only a resurfacing and other minor improvements.	\$41,100	Yes	This should be done.
F-3	Revise the design of the South Abutment to add a new substructure.	(-\$180,000)	Yes	Redesign is required to stabilize the existing abutment.
F-5	Close access on lower MLK, use current side road, Alabama Street.	\$89,800	No	ROW costs would exceed the proposed cost savings and would violate original ROW agreement. Loading dock access would be lost.

PSC Beams, Type III (H)				
H-1	Use BT-54 bulb tee PSC beams in lieu of BT-63 PSC units for spans 14 and 15.	Proposed= \$40,400 Actual= \$25,300	No	Cost savings reduced by \$15,100 due to additional pier concrete and reinforcement. Redesign costs would be approximately \$12,000.
H-2	Use smaller spans by reusing existing column locations in spans 1-5.	\$371,000	No	Would not allow access for trucks to use loading docks.

A meeting was held on March 6, 2009 to discuss the above recommendations. Ben Buchan, Nicoe Alexander, Jeff Simmons Gordon Sisk with Urban Design and Ron Wishon and Douglas Fadool with Engineering Services were in attendance. Additional information was provided by the Project Manager on March 13th and 19th, 2009.

The results above reflect the consensus of those in attendance and those who provided input.

Approved: Gerald M. Ross Date: 3/19/09
Gerald M. Ross, P. E., Chief Engineer

REW/DMF

Attachments

- c: Genetha Rice-Singleton
- Ben Buchan
- Chuck Hasty
- Nicoe Alexander
- Jeff Simmons
- Gordon Sisk
- Paul Liles
- Bill Ingalsbe
- Bill DuVall
- Doug Franks
- Melanie Nable
- James Magnus
- Mickey McGee
- Darrell Williams
- Percy Combay
- Ken Werho
- Lisa Myers
- General Files

DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA

INTERDEPARTMENT CORRESPONDENCE

FILE BHNLB-9073-00(016) & BRNLB-9073-00(018),
Fulton County
CS 999/Spring Street Viaduct Replacement over
CSX Railroad and Southern Railroad
P.I. Nos. 752086 & 752560

FROM *James B. Buchanan*
James B. Buchan, P.E., State Urban Design Engineer

TO Ron Wishon, Acting State Project Review Engineer
Attn: Lisa Myers

OFFICE Urban Design

DATE February 12, 2009



SUBJECT **Responses to Value Engineering Study**

The VE team's recommendations are noted below in italics and Urban Design's responses follow:

A-2 *Use a single Concrete box beam span for the CSX span pier 15-17.* There would be no cost savings associated with implementing this recommendation due to additional right of way impacts. Urban Design does not recommend implementing this alternative.

B-1 *Use three eleven foot wide travel lanes for Spring Street.* The proposed bridge needs to match the width of the existing bridge so there are no gaps between the proposed bridge and the adjacent buildings. The gaps associated with reducing the width of the bridge would negatively impact access from the bridge to the adjacent properties. Urban Design does not recommend implementing this alternative.

B-2 *Use one (1) 14 foot sidewalk on the west side in lieu of 2-10 foot sidewalks.* The City of Atlanta is requiring two (2), 10 foot sidewalks. Eliminating sidewalk on one side of the bridge would decrease safety for pedestrians using the Spring Street Bridge. Urban Design does not recommend implementing this alternative.

B-5 *Use a 6 inch sidewalk.* There would be not cost savings due to the costs associated with redesign of the superstructure. Urban Design does not recommend implementing this alternative.

B-6 *Use Class "B" concrete for bridge sidewalks.* Class "B" concrete is not acceptable for use for sidewalks on bridges. Urban Design does not recommend implementing this alternative.

C-1 *Design Consideration to use more, smaller diameter drilled caissons in lieu of larger units.* The Office of Material and Research has recommended caissons with a minimum diameter of 4-feet be used. The costs of drilling for additional caissons would exceed any perceived cost savings. Urban Design does not recommend implementing this alternative.

F-1 *Reduce work on Madison.* The work proposed for this area mainly requires resurfacing as suggested and would not require full depth pavement. The scope of the work on Madison can be reduced as much as possible to reduce project costs. Urban Design recommends implementing this alternative.

F-3 *Revise the design of the South Abutment to add a new substructure.* A new substructure may not be required and adding one in this area would significantly increase the cost of the project. After further discussion with the Office Bridge Maintenance, a meeting has been scheduled for March 3, 2009 in the Office of Bridge Design conference room to determine if the existing South Abutment can be reused to reduce project costs. The Office of Bridge Maintenance will assist in determining the most economical design to be used in this area.

F-5 *Close access on lower MLK.* During coordination meetings with project stakeholders, GSA requested access to Lower Martin Luther King Drive to their loading docks be maintained. Right of Way costs incurred by implementing this change would be higher than the cost savings during the construction phase. Urban Design does not recommend implementing this alternative.

H-1 *Use BT-54 bulb tee PSC beams in lieu of BT-63 PSC units for spans 14 and 15.* There would be no cost savings by implementing this recommendation. The costs associated with redesign of the superstructure would equal or exceed any estimated cost savings. Urban Design does not recommend implementing this alternative.

H-2 *Use smaller spans by reusing existing column locations in spans 1-5.* Access to GSA's loading docks will be improved by using longer spans. Reuse of existing column locations with longer spans is impossible. Urban Design does not recommend implementing this alternative.

If there are any further questions, please contact Jeff Simmons at (404) 631-1724 or Nicoe Alexander at (404) 631-1717.

^{SJA}
JBB:JNA:fgs

A-2 Use a single concrete box beam span for the CSX span pier 15-17.

Summary: We advise against adopting this recommendation.

We estimate a twenty-foot structure depth will be required for the 388 foot single span box girder. Whether or not there is adequate vertical clearance for the completed box girder over the railroad, the construction of a cast-in-place concrete box beam over CSX Transportation and the AJC building presents significant challenges. The railroad typically will not approve cast-in-place construction above active tracks. The falsework required to construct the box must be able to span the double tracks and provide adequate horizontal clearance, resulting in a falsework span of about 75 feet. Falsework will also have to be constructed above the AJC building, with a span of about 180 feet. The roof of the building is at approximately the same elevation as the tops of the existing columns, indicating approximately 15 feet of available depth above the AJC building. That is not enough room to accommodate the assumed structure depth, much less the falsework and formwork. The single column eliminated with this recommendation was carefully located to coordinate with proposed future rail lines in the area.

Not considered in the cost savings is the increased costs of piers 15 and 17 due to the additional dead load of concrete vs. steel and the increased loads resulting from the increased span lengths. The unit cost of \$240 per square foot for the concrete box girder option seems low, considering the significant site challenges and limited access.

If this recommendation is selected for implementation, the existing design and drawings will need to be revised at additional cost.

Concrete Box Girder (Assume \$1M for specialty falsework for the complicated construction to span over railroad and AJC Building):

$$= 24,409 \text{ SF} * \$240.00/\text{SF} + \$1,000,000$$

$$= \quad \quad \$ \quad 6,858,160.00$$

Assume a 40% cost increase in Piers 15 & 17 due to substantially increased loads:

$$\text{Pier 15} = 0.40 * [(176.4 \text{ CY} * \$850.00/\text{CY}) + (26327 \text{ LB} * \$1.01/\text{LB})]$$

$$= \quad \quad \$ \quad 70,612.11$$

$$\text{Pier 17} = 0.40 * [(139.6 \text{ CY} * \$295.81/\text{CY}) + (23236 \text{ LB} * \$1.01/\text{LB})]$$

$$= \quad \quad \$ \quad 25,905.37$$

ELEMENT	ORIGINAL ESTIMATE	VE ESTIMATE	REVISED ESTIMATE
2 SPAN STRUCTURAL STEEL	\$ 6,029,023.00	-	-
BENT 16	\$ 200,000.00	-	-
388' CONCRETE BOX GIRDER SPAN	-	\$ 5,858,160.00	\$ 6,858,160.00
BENT 15 ADDED COST	-	-	\$ 70,612.11
BENT 17 ADDED COST	-	-	\$ 25,905.37
SUBTOTAL	\$ 6,229,023.00	\$ 5,858,160.00	\$ 6,954,677.48
MARKUP 20%	\$ 1,245,804.60	\$ 1,171,632.00	\$ 1,390,935.50
TOTAL	\$ 7,475,000.00	\$ 7,030,000.00	\$ 8,346,000.00
SAVINGS		\$ 445,000.00	\$ (871,000.00)

B-1 Use three eleven foot wide travel lanes for Spring Street.

Summary: We advise against adopting this recommendation.

The proposed typical section for Spring Street north of the 1995 viaduct is 3 – 12'-0" lanes with 2'-0" gutters on both sides, for a gutter-to-gutter width of 40'-0". One of the four lanes on the 1995 construction will be a dedicated left turn lane into the CNN parking garage and "drops off" before the currently proposed construction. The overall bridge width cannot be reduced for spans 1 through 5 because the new bridge must be the same width as the existing bridge to fill the "hole" left when the existing bridge is removed. Reducing the width of the bridge by three feet could be accomplished by moving the east edge in between the end of the 1995 bridge and the AJC building, for a total length of 370 feet. This would create a discontinuity at pier 13 where the 1995 bridge ends and the proposed bridge begins. Once the roadway gets to the AJC building, the bridge could still be moved in, but approximately 200 feet of additional parapet will have to be added since the bridge will not be flush with the building. We would also have to provide a "bump out" for access to the AJC doorway on Spring Street.

Not considered in the cost savings is the additional cost for parapet at the AJC building and additional sidewalk width off the bridge. We would also disagree with using a square-foot cost as the basis for the savings. More cost savings is apparently realized by narrowing the steel spans, which is misleading. A three-foot reduction in bridge width will not significantly alter the design of the structural elements. Basing the cost savings on cubic yards of concrete may yield a more realistic answer.

If this recommendation is selected for implementation, the existing design and drawings will need to be revised at additional cost.

Saving 3'-0" of Deck based on a CY calculation yields a savings of:

$$\text{STA 20+38.78 TO 22+37.00} = (7.5''/12 * 3'-0'' * 198.22')/27 * \$1250.00/\text{CY}$$

$$\text{SAVINGS} = \$ 17,206.60$$

$$\text{STA 22+37.00 TO 26+25.00} = (9.25''/12 * 3'-0'' * 388.00')/27 * \$1250.00/\text{CY}$$

$$\text{SAVINGS} = \$ 41,539.35$$

$$\text{STA 26+25.00 TO 27+06.50} = (7.75''/12 * 3'-0'' * 81.50')/27 * \$1250.00/\text{CY}$$

$$\text{SAVINGS} = \$ 7,310.47$$

$$\text{STA 27+06.50 TO 27+77.00} = (7.5''/12 * 3'-0'' * 70.50')/27 * \$1250.00/\text{CY}$$

$$\text{SAVINGS} = \$ 6,119.79$$

Additional Parapet in front of AJC Building:

$$= 200' * \$218.21/\text{LF}$$

$$\text{SAVINGS} = \$ 43,642.00$$

Add sidewalk width off of bridge:

$$= 2 * (1/2 * 3' * 6' * 10''/12)/27 * \$1250.00/\text{CY}$$

$$\text{SAVINGS} = \$ 694.44$$

Connection to AJC Building:

$$= (3' * 10' * 10''/12)/27 * \$1250.00/\text{CY}$$

$$\text{SAVINGS} = \$ 1,157.41$$

ELEMENT	ORIGINAL ESTIMATE	VE ESTIMATE	REVISED ESTIMATE
BRIDGE DECK SP. 1-5	\$ 2,928,329.00	\$ 2,781,581.00	\$ 2,928,329.00
BRIDGE DECK SP. 14-19	\$ 11,323,259.00	\$ 10,783,442.00	\$ 11,251,082.78
ASPHALT	\$ 109,000.00	\$ 103,900.00	\$ 103,900.00
GAB	\$ 25,010.00	\$ 23,542.00	\$ 23,542.00
ADD'L PARAPET	-	-	\$ 43,642.00
ADD'L SIDEWALK	-	-	\$ 694.44
CONNECTION TO AJC	-	-	\$ 1,157.41
SUBTOTAL	\$ 14,385,598.00	\$ 13,692,465.00	\$ 14,352,347.64
MARKUP 20%	\$ 2,877,119.60	\$ 2,738,493.00	\$ 2,870,469.53
TOTAL	\$ 17,263,000.00	\$ 16,431,000.00	\$ 17,223,000.00
SAVINGS		\$ 832,000.00	\$ 40,000.00

B-2 Use 1-14 foot sidewalk on the west side in lieu of 2-10 foot sidewalks.

Summary: We advise against adopting this recommendation.

The City of Atlanta originally required the use of two ten-foot sidewalks and the decision was widely supported at various stakeholder meetings. Any decision regarding the sidewalk configuration should be made in conjunction with the City. Eliminating the sidewalk on the east side will force pedestrians to cross Spring Street at the end of the 1995 construction and again near the Georgia Bar Association. The Georgia Bar Association is on the east side of the roadway. The crossing at the Georgia Bar Association would happen within a substandard horizontal curve that has sight distance concerns; this presents an unsafe condition. Even if a formal crosswalk is not provided here, pedestrians may elect to jaywalk in a potentially unsafe location. Eliminating the sidewalk also cuts off pedestrian access to the AJC entrance on Spring Street. If sidewalk is not provided, the alternatives are a shoulder or a narrow curb and gutter in front of the rail. Either one could be used by pedestrians to walk along the road in unsafe conditions.

We disagree with using the per-square-foot cost of the steel spans as the basis for the savings. Basing the cost savings on cubic yards of concrete may yield a more realistic answer.

If this recommendation is selected for implementation, the existing design and drawings will need to be revised at additional cost.

B-2

Saving 6'-0" of Sidewalk based on a CY calculation yields a savings of:

$$\text{STA } 20+38.78 \text{ TO } 27+77.00 = ((8.5''(\text{avg. s/w thickness}))/12 * 6'-0'' * 738.22')/27 * \$600.00/\text{CY}$$

$$\text{SAVINGS} = \$ 69,720.78$$

ELEMENT	ORIGINAL ESTIMATE	VE ESTIMATE	REVISED ESTIMATE
SIDEWALK	\$ 11,323,259.00	\$ 10,243,624.00	\$ 11,253,538.22
SUBTOTAL	\$ 11,323,259.00	\$ 10,243,624.00	\$ 11,253,538.22
MARKUP 20%	\$ 2,264,651.80	\$ 2,048,724.80	\$ 2,250,707.64
TOTAL	\$ 13,588,000.00	\$ 12,292,000.00	\$ 13,505,000.00
SAVINGS		\$ 1,296,000.00	\$ 83,000.00

B-5 Use a 6 inch thick sidewalk.

Summary: We advise against adopting this recommendation.

The roadway profile, cross-slopes, and superelevation are currently set to provide access to adjacent facilities with an assumed 7-inch sidewalk. Implementing this recommendation would require revising the roadway profile, cross-slopes and superelevation to ensure smooth tie-ins. If this recommendation is selected for implementation, the existing design and drawings will need to be revised at additional cost.

B-6 Use Class B concrete for bridge sidewalks.

Summary: We have no opinion regarding the adoption of this recommendation.

This suggestion could be implemented at the direction of GDOT. It has not been the policy of the bridge office to use a different strength concrete for the sidewalk.

C-1 Design Consideration to use more, smaller diameter drilled caissons in lieu of the larger units.

Summary: We advise against adopting this recommendation.

GDOT policy is to use caissons with a minimum diameter of four feet in order to facilitate downhole inspections. Adding caissons to provide design flexibility will result in significant additional cost. It is not significantly cheaper per linear foot to drill a four foot diameter caisson than it is to drill a seven foot diameter caisson. Most of the caissons on the project are five or five-and-a-half feet in diameter, and using more smaller caissons will not result in an optimized design. Four-foot diameter caissons have to be at least 10 feet apart. Providing the required spacing may necessitate constructing footings for use as "pile caps," adding even more cost to the project. Subsurface Utility Engineering is a part of this project. Test holes are proposed to be drilled at any proposed caisson location that may interfere with existing underground utilities, minimizing the potential for unknowns during construction.

If this recommendation is selected for implementation, the existing design and drawings will need to be revised at additional cost.

F-1 Reduce work on Madison Avenue.

Summary: We support adopting this recommendation.

We concur with the recommendation to resurface Madison Street south of MLK Jr. Drive in lieu of reconstruction.

F-3 Revise the design of the South Abutment to add a new substructure.

Summary: Further investigation is needed into the design of the south abutment.

The design of the south abutment may need to be revised in light of recent decisions by the Office of Bridge Maintenance. The design proposed in the VE study is not a workable solution as the existing south abutment is not self-supporting. Further investigation of a design solution is needed here. The necessary re-design of the South abutment is outside of the current scope of work.

F-5 Close access on lower MLK.

Summary: We have no opinion regarding the adoption of this recommendation.

This decision would have to be made by GDOT in coordination with GSA.

H-1 Use BT-54 bulb tee PSC beams in lieu of BT-63 PSC units for spans 14 and 15.

Summary: We advise against adopting this recommendation.

The original design of this bridge utilized Type V beams, which are no longer available, and a maximum concrete strength of 5 ksi, per the City of Atlanta. BT-63 beams were substituted to provide the same structure depth, allowing the use of the existing beam seats at pier 13 (constructed in 1995) without modification. If this recommendation is accepted, the beam seats at pier 13 will need to be built-up by nine inches to accept the shallower beams. The beams will need to be redesigned and re-detailed. The substructure design will need to be verified for the modified loads and taller columns. The cost savings is not significant, when the cost of re-design and modifications to bent 13 are considered.

Additional Concrete on Pier 13 (CLASS AA-1):

$$= (2.75' * 67'-3 \frac{1}{2}" * 9"/12)/27 * \$850.00/CY$$

$$= \qquad \qquad \$ \qquad \qquad 4,369.29$$

Additional Concrete at Pier 14 (CLASS AAA):

$$= (18' * 5.5' * 9"/12)/27 * \$850.00/CY$$

$$= \qquad \qquad \$ \qquad \qquad 2,337.50$$

Additional Concrete at Pier 15 (CLASS AAA):

$$= (2.75' * 62.25' * 9"/12)/27 * \$850.00/CY$$

$$= \qquad \qquad \$ \qquad \qquad 4,041.93$$

Additional Reinforcement (avg. Piers 14 & 15):

$$= ((23613 \text{ LB}/183.8 \text{ CY}) + (26327 \text{ LB}/176.4 \text{ CY}))/2$$

$$= \qquad \qquad \qquad 139 \text{ LB}/\text{CY}$$

$$= [(2.75' * 67'-3 \frac{1}{2}" * 9"/12) + (18' * 5.5' * 9"/12) + (2.75' * 62.25' * 9"/12)]/27$$

$$= \qquad \qquad 13 \text{ CY} * 139 \text{ LB}/\text{CY} * \$1.01/\text{LB}$$

$$= \qquad \qquad \$ \qquad \qquad 1,825.07$$

ELEMENT	ORIGINAL ESTIMATE	VE ESTIMATE	REVISED ESTIMATE
2 SPANS -63" BULB TEE	\$ 333,441.00	-	-
2 SPANS -54" BULB TEE	-	\$ 299,754.00	\$ 299,754.00
ADD'L PIER CONCRETE COST	-	-	\$ 10,748.71
ADD'L PIER REINFORCEMENT COST	-	-	\$ 1,825.07
SUBTOTAL	\$ 333,441.00	\$ 299,754.00	\$ 312,327.78
MARKUP 20%	\$ 66,688.20	\$ 59,950.80	\$ 62,465.56
TOTAL	\$ 400,100.00	\$ 359,700.00	\$ 374,800.00
SAVINGS		\$ 40,400.00	\$ 25,300.00

H-2 Use smaller spans by reusing existing column locations in Spans 1-5.

Summary: We advise against adopting this recommendation.

This recommendation appears to be in conflict with the Office of Bridge Maintenance directive not to re-use any part of the existing bridge. Maintaining the existing column location does not allow access to the loading dock at the MLK building. Encasing the columns will add about 3 feet to the width of the columns in both directions, further restricting access.

**DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA**

INTERDEPARTMENT CORRESPONDENCE

FILE **BHNLB-9073-00(016)FULTON** OFFICE Atlanta, GA
Spring St. over CSX Transportation
P.I. 752086 DATE March 6, 2009

FROM Paul V. Liles, Jr., State Bridge Engineer

TO Ben Buchan, State Urban Design Engineer

ATTENTION: Nicoe Alexander

SUBJECT **V.E STUDY RECOMMENDATION**

One of the value engineering study recommendations is to utilize Class "B" concrete in the bridge sidewalks in lieu of Class "AA" concrete as proposed in the plans. The Office of Bridge Design does not recommend using the Class "B" concrete because the water/cement ratio is higher(.660) compared to Class "AA"(.445) and a has a lower limit of air entrainment which would result in a surface which is not as durable as Class "AA". The cost savings shown on the ve study would not be obtained due to class "B" is typically not reinforced with rebar and the bridge sidewalks require reinforcing. The cost of the rebar and labor would drive up the cost of using Class "B" concrete.

If you have any questions please contact Steve Wyche of the Bridge Office at 404-656-5289.

PVL:SWW

cc: Bill DuVall, attn: Steve Wyche

PRECONSTRUCTION STATUS REPORT FOR PI:752086-

PROJ ID : 752086- SR 999/CS 3586/SPRING STREET OVER CSX RAILROAD
COUNTY : Fulton
LENGTH (MI) : 0.50
PROJ NO.: BHNLB-9073-00(016)
PROJ MGR: Alexander, Nicoe
OFFICE : Urban Design
CONSULTANT: Turnkey Consultant, (Contract with GDOT)
SPONSOR : Atlanta
DESIGN FIRM: Heath & Lineback Engineers, Inc.

MGMT LET DATE : 07/15/2011
MGMT ROW DATE :
DOT DIST: 7
CONG. DIST: 5
BIKE: Y
MEASURE: E
NEEDS SCORE: 9
BRIDGE SUFF:

SCHED LET DATE : 7/4/2011
WHO LETS? : GDOT Let
LET WITH : 752560-

SCHED		ACTIVITY	ACTUAL		%	PROGRAMMED FUNDS				STIP AMOUNTS			
START	FINISH		START	FINISH		Approved	Proposed	Cost	Fund	Status	Date Auth	Phase	Cost
		Concept Development	1/1/1996	11/10/1997	100	1993	1993	136,250.52	Q10	AUTHORIZED	8/10/1992		
		Concept Meeting	4/14/1997	4/14/1997	100	2003	2003	2,000,000.00	Q10	AUTHORIZED	6/26/2003		
		PM Submit Concept Report	7/30/1997	7/30/1997	100								
		Receive Preconstruction Concept Approval	9/4/1997	9/4/1997	100								
		Management Concept Approval Complete	10/10/1997	11/10/1997	100	2012	2012	3,879,895.95	LICO	PRECAST			
4/28/2009		Value Engineering Study	8/27/2008		64								
		Public Information Open House Held	8/12/1996	1/12/1999	100								
		Environmental Approval	1/11/1998	10/4/2000	98								
		Pub Hear Held/Comm Resp (E/A/FONSI, GEPA)	10/4/2000	10/4/2002	100								
		Field Surveys/SDE	3/9/1993	4/5/1993	100								
		Preliminary Plans	11/15/1997	2/19/1998	100								
		Preliminary Bridge Design	11/15/1997	2/19/1998	100								
		Underground Storage Tanks	1/9/2003	7/30/2003	100								
		FPFR Inspection	3/27/1998	3/27/1998	100								
		R/W Plans Preparation	10/10/2001	11/1/2001	100								
		R/W Plans Final Approval	10/10/2001	11/1/2001	100								
		L & D Approval	9/11/2001	9/13/2001	100								
3/27/2009		R/W Acquisition			14								
7/2/2009		Stake R/W			0								
		Soil Survey	2/5/2003	3/20/2003	100								
		Bridge Foundation Investigation	2/14/2003	9/26/2008	100								
3/27/2009		Final Design			0								
5/20/2009		Final Bridge Plans Preparation			0								
3/1/2011		FPFR Inspection			0								
3/16/2011		Submit FPFR Responses (OES)			0								

PDD: [01C] 12-30-99 CITY WANTS ADVANCE CONSTR DOLLARS. SEE 752560 FOR ANOTHER SPRING ST PROJ. 10/28/96 /AOE
Bridge: SWW 02/04/08 CONSULT-H&L
Design: NA:FGS:H&L VE Responses sent 2/16/09 090220
EIS: CE/Apvd 10-4-00/RE 5-21-03/OnSchedule/INtable(3.5.08)
LGA: REV PMA SGN ATLANTA DO PE & UTIL 3-6-02.
Planning: SR 999/CS 3586/Spring St. over CSX Railroad is on the Atlanta Commuter On-Street Bike Plan (1995) map and pg 76
Programming: TEMP SR 999 & 999TA#1 9-07/#2 2-08
ROW: plan issues and arty. issues. assign. D7 ROW 6-08; Pre-Acq. Payne, Rubio (CC)
Railroad: CSX
Traffic Op: DDC I BR REPL. PRCTJ/S&M PLNS N/R0320011\$
Utility: RC:SUE Level B Extended 12/17/07 ongoing-SO-Deep
EMG: BRIDGE REPLACEMENT: #1363 (CONTROL SURVEY)

District Comments
 COA, SUFF RATING = 4 (8/21/02) PROJECT DELAYED BECAUSE OF R/W. EXPECTED TO GO TO FY 2004 CST. (8/12/03) COMPLICATED R/W. WORKING WITH GSA ON PARKING. (10/9/03) WORKING FINAL PLANS. CITY CONSULTANT AWAITS COST PROPOSALS TO COMPLETE DESIGN. (8/12/04) CITY WORKING ON PLANS. WILL NEED TO CONVERT FILES. (11/15/04) RECOMM. 7/07 LET. NEED RE-EVAL. (4/14/05) R/W ACQUIS. ON HOLD UNTIL NEW CONSULTANT CONTRACT IS COMPLETE. (12/8/05) COA NEEDS TO SUBMIT A REQ. TO DEPARTMENT ASKING FOR ASSISTANCE TO COMPLETE PLANS.
 GO ROW Comments: Rec'd R/W plan revs.3/01/08; Appr:Cont Pending

Prel. Parcel CT: 2	Total Parcel in ROW System: 7	Cond. Filed: 0	Acquired by: DOT	DEEDS CT: 0
Under Review: 0	Options - Pending: 0	Relocations: 0	Acquisition MGR: Phillips, Sherry	
Released: 0	Condemnations- Pend: 0	Acquired: 0	R/W Cert Date:	

PRECONSTRUCTION STATUS REPORT FOR PI:752560-

MGMT LET DATE : 07/15/2011
MGMT ROW DATE :
SCHED LET DATE : 2/16/2011
WHO LETS? : GDOT Let
LET WITH : 752086-

DOT DIST: 7
CONG. DIST: 5
BIKE: Y
MEASURE: E
NEEDS SCORE: 8
BRIDGE SUFF: 3.00

MPO: Atlanta TMA
TIP #: AT-086B
MODEL YR : 2020
TYPE WORK: Bridges
CONCEPT: BR REPL
PROG TYPE: Replacement
BOND PROJ :

PROJ ID : 752560-
COUNTY : Fulton
LENGTH (MI) : 0.48
PROJ NO.: BRNLB-9073-00(018)
PROJ MGR: Alexander, Nicole
OFFICE : Urban Design
CONSULTANT: Turnkey Consultant, (Contract with GDOT)
SPONSOR : Atlanta
DESIGN FIRM: Heath & Lineback Engineers, Inc.

SCHED		ACTIVITY	ACTUAL		%	PROGRAMMED FUNDS				STIP AMOUNTS		
START	FINISH		START	FINISH		Phase	Approved	Proposed	Cost		Fund	Status
3/27/2009	4/28/2009	Concept Development	1/1/1996	11/10/1997	100	PE	1998	1998	858,653.90	Q10	AUTHORIZED	2/6/1998
		Concept Meeting	1/1/1996	11/10/1997	100	ROW	2003	2003	5,013,500.00	Q10	AUTHORIZED	6/26/2003
		PM Submit Concept Report	1/1/1996	11/10/1997	100	CST	2012	2012	16,975,413.00	LICO	PRECAST	
		Receive Preconstruction Concept Approval	1/1/1996	11/10/1997	100							
		Management Concept Approval Complete	8/27/2008	11/10/1997	100							
		Value Engineering Study	8/12/1996	1/12/1999	64							
		Public Information Open House Field	3/1/1999	10/4/2000	100							
		Environmental Approval	10/4/2002	10/4/2002	100							
		Pub Hear Held/Comm Resp (EA/FONSI, GEPA)	3/9/1993	4/5/1993	100							
		Field Surveys/SDE	10/15/2001	8/10/2003	75							
		Preliminary Plans	2/23/2003	12/12/2003	67							
		Preliminary Bridge Design	4/29/2000		100							
		Underground Storage Tanks	3/27/1998	3/27/1998	0							
		404 Permit Obtainment	10/29/2001	11/1/2001	100							
		PFPR Inspection	10/29/2001	11/1/2001	100							
		R/W Plans Preparation	10/29/2001	11/1/2001	100							
		R/W Plans Final Approval	9/11/2001	9/13/2001	100							
		L. & D Approval			14							
		R/W Acquisition			0							
		Stake R/W	12/30/2007		55							
		Soil Survey	12/31/2007		0							
		Bridge Foundation Investigation	12/28/2007	9/26/2008	100							
		Final Design			25							
		Final Bridge Plans Preparation			0							
		FFPR Inspection			0							
		Submit FFPR Responses (OES)			0							
3/27/2009	7/30/2009	Underground Storage Tanks										
7/2/2009	6/11/2009	PFPR Inspection										
		12/21/2010										
		7/15/2009										
		3/30/2010										
		1/7/2010										
		12/31/2009										
		2/1/2010										
		2/26/2010										

Phase PE ROW CST
Cost 0.00
Fund Q10 Q10 LICO
Date: 1/6/2009
Date:
Date:

District Comments
 COA., UST N/R - GOES W/752086. (8/2/02) PROJECT DELAYED BECAUSE OF R/W - URBAN DESIGN TO RECOM. FY 2004 CST. (8/21/02) PROJECT DELAYED BECAUSE OF R/W. EXPECTED TO GO TO FY 2004 CST. (8/14/03) COMPLICATED R/W - WORKING WITH GSA ON PARKING. (9/22/03) MOVED TO JULY 05 LET. (2/12/04) NO ACTIVITY. (6/10/04) CITY STILL SHORT ON PE FUNDS. PROJECT NEEDS TO GO IN FY 2006 CST. (11/15/04) WORKING ON ENV. DOC. RECOMM. 3/06 LET. (12/8/05) COA NEEDS TO SUBMIT REQ TO DEPARTMENT ASKING FOR ASSISTANCE TO COMPLETE PLANS.
 GO ROW Comments: appraisal contract issues....
Acquired by: DOT
Acquisition MGR: Phillips, Sherry
R/W Cert Date:

DEEDS CT: 0

